OBSERVATION AND SYNTHESIS OF SPATIALLY-INCOHERENT WEAK-MOTION WAVEFIELDS AT ALFREDTON BASIN, NEW ZEALAND

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SUMMARY

To observe and model the detailed pattern of ground motion amplification in a small soft-soil basin an experiment was conducted at Alfredton, New Zealand. 19 seismometers were deployed for 5 weeks at closely spaced sites in and around a 400-500 m diameter, sediment-filled depression in soft, sandstone basement. During this period 112 earthquakes, with "weak" ground motions, were detected by at least some of the instruments, and 15 well-recorded events were selected for detailed analysis. Geotechnical data obtained to provide the parameters for the 3-dimensional modelling included measurements of the shear-wave velocity. Across the basin this is 60 m/s at the surface, increasing steadily to 300+ m/s at the bottom of the basin, and the shear-wave velocity in the basement is 850 m/s. Thus, there are no boundaries where the contrast in shear-wave impedance is especially large.

In contrast to situations where there are large contrasts in shear-wave impedance to trap seismic energy in soft-soil layers, the amplifications observed in the basin at Alfredton were small. The small amplifications are confirmed by the 3-dimensional modelling. Another feature of the observed wavefields is that in all cases the incident motions, recorded at the basement sites around the basin, were spatially incoherent. In other words, the wavefields arriving at the basin were of a complex, seemingly random nature. This is the first occasion that the spatial coherency of wavefields has been measured in a fine-scale experiment in New Zealand. Apart from the small amplifications and the observed lack of coherency between the basement sites, the most striking result, which was obtained from both the observations and the modelling of similarly incoherent wavefields, is that for short-duration events in which the main motions last for no more than a second, the amplifications in the basin are larger than for events in which the motions are of longer duration; that is, the extent to which differently propagating incoherent wave packets interfere destructively inside the basin increases with the duration of the wavefields.

INTRODUCTION

In this paper we present observations and modelling results from an experiment at a small basin near Alfredton, northern Wairarapa, New Zealand. 19 identical portable seismographs were deployed in and around the basin for 5 weeks in 1993, during which 112 earthquakes were recorded, ranging in size from less than magnitude 2 to magnitude 5. The purpose of the experiment was to identify the detailed pattern of ground motion amplification in the basin, as a guide to what can be expected in similar basins, and a secondary goal was to verify a recently developed package for modelling 3-dimensional seismic wave amplification in complex near-surface structures. To characterise the basin and provide physical parameters needed for the modelling, detailed geotechnical data were acquired. The basin infill consisted of up to 16 m of recent, fine-grained, water-saturated sediments, with shear-wave velocities increasing steadily with depth from 60 m/s at the surface to 300+ m/s, underlain by sandstone basement, with a low shear-wave velocity of 850 m/s. As the results we present show, the lack of any boundary at which there is a very large abrupt change in shear-wave impedance, results in the seismic wave amplification being small compared with basins where large proportions of the incident seismic energy become trapped within the low-velocity layers. Another important feature of the results is that in no case did the incident S wavefield contain simple coherent wavefronts. Instead, all the incident S wavefields consisted of spatially-incoherent arrivals, probably due to scattering of the seismic waves by heterogeneities in the crust below the basin. Though this has meant that we were unable to verify the modelling using instances of precisely-known incident wavefields, we have reproduced statistical features of the observed amplifications, by modelling the response of the basin for ensembles of incident S wavefields with similar time histories and random spatial distributions to the observed incident wavefields.

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the shear-wave speed of 850 m/s, very much lower than the wave speeds in hard rock, the half-

The very low shear-wave velocities of the soft soil in Alfredton basin are typical of sites where very large amplifications have been observed. New Zealand examples include the basins at Porirua and Wainuiomata and the area of marine sediments overlying the gravels in the Hutt Valley (Taber and Smith, 1992), the vicinity of the new Museum of New Zealand site on the Wellington waterfront (Taber, 1993), areas of thick silt and peat in central Christchurch (Taber and Cowan, 1993), and river and marine sediments in Wairoa and downtown Gisborne (Barker, 1994). The classic example worldwide is the area of fine-grained deposits in what was a lake 500 years ago covering much of the present-day heart of Mexico City, and a close analogue to this in New Zealand is at Wainuiomata where a similar lake existed until recent times (Stephenson and Barker, 1992; Begg et al., 1993). Amplifications at such sites are generally restricted to parts of the incident wavefields in specific, narrow frequency bands determined by the local site conditions, and the amplification factors in these frequency bands range from, say, 10 to 40 or more, relative to the corresponding wavefields at basement rock sites. As we will show, the amplifications in Alfredton basin are much smaller than this, because of the lack of a boundary where there is a large contrast in shear-wave impedance. Our results in this regard are in accord with calculations for comparable 1-
dimensional models by Davis (1995).

On the other hand, the amplifications at Alfredton are larger than have been typically observed in strong motion at soil sites throughout New Zealand. Zhao et al. (in prep.) have found, for example, that the peak ground acceleration at sites with soil more than 3 m thick are only 41 percent stronger on average than at other sites. In comparison, for sites from each class the individual peak ground accelerations are significantly more variable with the standard deviation being 1.5 times the average difference between the two classes. We will show that the variability in peak ground acceleration between the closely spaced basement sites at Alfredton is only slightly smaller than that found by Zhao et al. (in prep.) for much more widely spaced sites.

The other outstanding feature of the wavefields observed at Alfredton is the spatial incoherence seen at the basement sites which surrounded the basin. Observations of the coherence of incident wavefields have been made in few places worldwide, and most observations that have been made have been in specially chosen, hard rock areas where closely spaced arrays of seismometers have been deployed so that the testing of nuclear weapons can be monitored globally. In other words, these areas were selected because the incident wavefields were likely to be as simple, coherent and easy to interpret as was thought possible. Even so, it is not unusual for the wavefields to be incoherent at sites separated by more than half a wavelength, as we find to be the case at Alfredton for the small earthquakes which were recorded, and the major cause of the incoherence is known to be scattering of the incident waves by heterogeneities in the crust in the vicinity of the recording sites (Wu and Aki, 1985). Alfredton is the first experiment in New Zealand where spatial coherency has been tested on a similar scale. Because the shear-wave speed of 850 m/s in the basement at Alfredton is very much lower than the wave speeds in hard rock, the half-

wavelength distances over which the wavefields are coherent at any given frequency, are much smaller. Other sites in New Zealand where the basement rock is likely to be similarly “soft” are distributed throughout the North Island, where much of the near-surface material is little more than compacted mud.

THE ALFREDTON EXPERIMENT AND PHYSICAL PROPERTIES OF THE BASIN

The site at Alfredton in the southern part of the North Island, New Zealand (Figure 1), is a small, roughly round sediment-filled depression 400-500 m in diameter (Figure 2). The basin is bounded to the north by the Alfredton fault which has affected the flow of the Ihuraua River, to the west by a hill, to the east by highway 52 at the base of another hill, and to the south the basin widens out into a much larger, less boggy area of recent sediments.

The 19 identical EARSS portable seismographs from GNS (Institute of Geological & Nuclear Sciences) and Institute of Geophysics, Victoria University of Wellington, were deployed at over 20 sites, and two P wave, seismic refraction profiles spanning the basin north-south and east-west (Figure 3). In addition, samples were taken of soft soil for laboratory measurement of water content and plastic limits and other properties for the wavefield modelling of the “weak ground motion” were the basement S wave velocities, interpolated between the SCPT sites.

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FIGURE 1. Location of Alfredton basin and the 15 earthquakes analysed in this study.
FIGURE 2. Location of seismometer sites. A00, A05, A11 and A12 are firm, basement sites. The square with the dashed outline is the area for which the geometry of the basin is shown in Figure 5.

FIGURE 3. Location of penetrometer probes and seismic refraction profiles. Stars and circles denote respectively cone penetrometry (CPT) and seismic cone penetrometry (SCPT), which includes cone penetrometry as well. Solid lines mark the P wave seismic refraction profiles L1 and L2, and the square with the dashed outline is the area for which the geometry of the basin is shown in Figure 5.
FIGURE 4. S wave travel times versus depth for the 8 sites at which seismic cone penetrometry data were acquired (from Stephenson, 1993). The shear-wave velocities, which are the gradients of the lines, are smoothly increasing with depth.

FIGURE 5. The geometry of the basin used in the wavefield modelling, showing the top and bottom surfaces, viewed from the south-southeast. The vertical exaggeration is 5:1. This is the area with the dashed outline in Figures 2 and 3.

THE OBSERVATIONS

In summarising the seismic wave observations we will focus first on aspects of the observed peak ground motions that are indicative of the incoherence of the incident wavefields and the patterns of amplification in the basin. We will show that two distinct patterns of amplification were observed. For the three events 8, 9 and 15 in Figure 1 that were closest to Alfredton the amplification in the basin was significantly larger than for the remaining 12 events we have studied in detail. In situations where there are large enough abrupt changes in shear-wave impedance to trap seismic energy in low-velocity, soft-soil layers, the duration of the ground shaking (number of cycles) is generally markedly increased in these layers, as well as there being increases in the amplitudes of the S waves (Borcherdt and Glassmoyer, 1992; Gutierrez and Singh, 1992). In contrast, at Alfredton we observed no noticeable difference in duration between the motions in the basin and the motions at the four firm reference sites. We will illustrate that for the three events for which the amplification was largest the duration of shaking was very short, both at the reference sites and in the basin. Then for events in both this set and the other set we will show that the incident wavefields were incoherent except at low frequencies for which the diameter of the basin is less than half the wavelength of S waves in the basement, much as has been observed elsewhere. We will conclude this section on the observations by presenting spectral ratios relative to the firm sites for a representative set of basin sites. These ratios illustrate that the spectral amplifications in the basin occur at frequencies higher than those at which the incident wavefields in the basement are coherent across the whole base of the basin.

The S wave observational data presented in this paper, and by Yu and Haines (1994) and Yu (1996), are based on true ground motions recovered from the seismic recordings by allowing for the response of the instruments. However, all the results we will show have been normalised in some way, since the absolute levels of the motions are not important, given that the medium will have behaved linearly for the weak-motion strains involved. Unless we state otherwise, this will be with respect to the motion at the set of the four firm reference sites A00, A05, A11 and A12 that recorded each event. Because of the incoherence of the incident wavefields, we have had to normalise with respect to an average of the recordings at the reference sites, which we have generally chosen to be the appropriate RMS (root-mean-square) value.

Figure 6 shows the standard deviations of each of various measures of peak ground motion at the firm reference sites for each event, normalised with respect to the corresponding RMS value – D, V and A denote displacement, velocity and acceleration, and Z, E, N and H denote the vertical, east-west and north-south components and the maximum horizontal motion respectively. For comparison, for sites throughout New Zealand in separate rock and soil classes the corresponding standard deviation of peak ground acceleration found by Zhao et al. (in prep.) is equivalent to a value of 0.49 in Figure 6. It can be seen in Figure 6 that the differences between the peak values at the firm reference sites are greater for velocity than for displacement, and generally greatest for acceleration, for which the standard deviations approach the value found by Zhao et al. (in prep.) for much more widely spaced sites. This difference in variability between displacement, velocity and acceleration is in accord with the coherence of the incident wavefields decreasing with increasing frequency, given that peak displacement, velocity and acceleration are sensitive to the spectral amplitudes at progressively higher frequencies.
FIGURE 6. Standard deviations of the peak ground motion values at the firm sites recording each event. PGD = peak ground displacement, PGV = peak ground velocity, PGA = peak ground acceleration. \( Z \) = vertical component, \( E \) = east-west component, \( N \) = north-south component, and \( H \) = maximum horizontal motion. In each case, the normalisation is with respect to the RMS (root-mean-square) value, which is always greater than the standard deviation.

Table 1. Stations recording each event

<table>
<thead>
<tr>
<th>Event</th>
<th>1</th>
<th>2</th>
<th>3</th>
<th>4</th>
<th>5</th>
<th>6</th>
<th>7</th>
<th>8</th>
<th>9</th>
<th>10</th>
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<th>12</th>
<th>13</th>
<th>14</th>
<th>15</th>
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</thead>
<tbody>
<tr>
<td>Lat S</td>
<td>40.64</td>
<td>41.06</td>
<td>37.92</td>
<td>38.99</td>
<td>38.44</td>
<td>40.42</td>
<td>40.04</td>
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<td>40.15</td>
<td>41.32</td>
<td>40.08</td>
<td>38.63</td>
<td>40.24</td>
<td>40.86</td>
</tr>
<tr>
<td>Long E</td>
<td>174.80</td>
<td>175.92</td>
<td>178.20</td>
<td>174.87</td>
<td>175.91</td>
<td>175.64</td>
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<td>176.04</td>
<td>176.54</td>
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<td>107</td>
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<td>197</td>
<td>40</td>
<td>143</td>
<td>104</td>
<td>30</td>
</tr>
<tr>
<td>Mag</td>
<td>3.7</td>
<td>3.1</td>
<td>5.9</td>
<td>4.0</td>
<td>4.3</td>
<td>3.7</td>
<td>4.0</td>
<td>2.9</td>
<td>3.8</td>
<td>4.6</td>
<td>3.9</td>
<td>5.1</td>
<td>4.1</td>
<td>4.0</td>
<td>2.6</td>
</tr>
</tbody>
</table>

Notation in the table:

- PS P and S waves recorded
- P P waves recorded
- S S waves recorded
- * No recording
- Not in operation
values are shown for event 14 because A12 was the only firm site that recorded that event (see Table 1). By normalising with respect to averages from all the reference sites for the other events, rather than with respect to the values at one site, say A12, we have obtained peak-motion and spectral amplifications in the basin that are significantly more consistent between events than they would have been otherwise.

Figure 7 summarizes the normalised east-west and north-south horizontal peak ground displacement, for (a) the nearby events 8, 9 and 15 and (b) the more distant events. Three ellipses are shown at each site, with axes in the east-west and north-south directions. Separate values are presented for the three special, local events and for the other, more distant events. The middle ellipse gives the mean normalised peak displacements in the two directions, averaged over the events recorded at each site, and the outer and inner ellipses are plus and minus one standard deviation.

As a guide in interpreting these results and the spectral ratios that are presented later, the following basic concepts are helpful. If absolutely none of the energy that enters the basin became trapped there (in the sense described below) and the incident S wavefields were completely coherent, then the simplistic ray-theory approximation for high-frequency wavefields (Aki and Richards, 1980) gives that the amplitudes in the basin would be about 3.5 times the amplitudes at the firm sites. This approximation for situations where there are no abrupt changes in material properties is exactly the same as the high-frequency asymptote obtained by Davis (1995) for 1-dimensional models of this type that involve no damping. The difference in wavefield amplitude between soft and firm sites is dictated in such cases solely by the difference in material properties and is necessary for the energy flux to be the same, which depends on the product of the seismic impedance with the square of the amplitude of the wavefield - for S waves the impedance is the product of the shear-wave velocity and the density, which is about 1.85 g/cm³ in the basin and 2.2 g/cm³ in the basement (Stephenson, 1993). If, instead, a large fraction of the energy was trapped in the basin for a long duration, the ratios of the peak amplitudes and, in particular, the spectral amplitudes at the dominant frequencies for the basin would be considerably greater than this value of 3.5.

To clarify what we mean by “trapped” in the context we are considering, let us follow what happens to seismic energy after it has entered the basin. Initially the energy will propagate up to the surface. At the surface it will be reflected downwards. Then when it reaches the base of the basin, or any internal discontinuity where the impedance changes abruptly, a fraction of the energy will be reflected back upwards. How large this fraction is depends on the magnitude of the contrast in impedance at the discontinuity. If the contrast is very large, then almost all of the energy is reflected back upwards, which is why the word “trapped” is used. On the other hand, if the impedance below the discontinuity is, say, three times or twice the impedance above the discontinuity, much as is the case at Alfredton, the fraction of the energy that is reflected upwards is only one quarter or one ninth respectively. This process is then repeated until all the energy trapped in the basin has escaped.

The results in Figure 7 show that on the whole, rather than being greater than or equal to 3.5, the normalised values of the peak amplitudes in the basin are more typically about 2. Part of the explanation for this might be that weathering of the exposed basement material near the ground surface has resulted in the S wave velocity at the firm sites being less than the value
of 850 m/s obtained from cores taken from under the basin. This would have the effect that the difference in amplitude between the firm sites and the basin would be smaller. All the same, it is clear that at most a modest fraction of the incident energy can be being trapped in the basin. The pattern of peak velocities is almost identical to that in Figure 7 for the peak displacements, whereas the normalised values of peak acceleration are more variable at each site, and are less systematically distributed between sites, though overall the normalised values of peak acceleration inside the basin are marginally smaller than the normalised values of peak displacement and peak velocity (Yu and Haines, 1994; Yu, 1996).

The most striking feature in Figure 7 is the markedly larger relative amplitudes in the basin for the north-south component for the three nearby events 8, 9 and 15. For these events the mean normalised peak displacements in the north-south direction of up to a value of 4 in the centre of the basin, were twice the values in the east-west direction. As we illustrate below, the distinguishing feature of the three events is the very short duration of the S wavefield. In the discussion of the modelling results in the next section we show that for incoherent incident wavefields with similarly short durations, the peak displacements in the basin are generally larger than for incoherent incident wavefields of longer duration. However, the modelling results predict that averaged over a large number of distinct events there should be no noticeable difference in this behaviour between the north-south and east-west components. Given that events 8, 9 and 15 were very close together, it is possible that they were also very similar in all other regards, including their source mechanisms and rupture characteristics; that is, they may have been almost identical events, that just happened to give larger amplitudes in the basin in the north-south direction than the east-west direction, but there is no way of verifying this satisfactorily.

A point to note with regard to the different amplifications for events 8, 9 and 15 compared with the other events is that angle of incidence is most unlikely to have been a major factor in this difference. Because of the very low S velocity of 850 m/s in the basement below the basin, the angles of incidence for the basic S-wave arrivals for all of the events would have been no more than 10–15° from vertical. Consequently, the fact that events 8, 9 and 15 were close to being directly beneath the basin is not especially significant.

Figure 8 shows S wave horizontal ground displacements in one second segments for the largest nearby event 9 (magnitude 3.7) and event 12 (magnitude 5.1) which had the largest ground motions of the more distant events. The displacement paths are shown for the firm site A12 and two representative sites A02 and A15 in the basin, and are normalised to the largest displacements, which are at site A02. The short duration of the main motion in event 9, lasting no more than a second, is particularly clear. It is also obvious that, apart from occurring at the same times, the motions at the three sites are similar only in that they all appear to be completely random.

The coherence of the wavefields at the firm sites is quantified for the same two events in Figure 9. Coherence values close to 1.0 imply that the wavefields are effectively the same at all of the firm sites (Yu, 1996). Values are given as a function of frequency for overlapping, 6-second long time windows, starting

FIGURE 8. Horizontal ground displacements in events 9 and 12. Each window shows a track for one second.
from the time of arrival of the S waves. Each successive window begins 3 seconds after the start of the preceding window. A remarkable feature of the values in Figure 9 is that, as is true of all events, except the deep distant events 4, 5, 11 and 13, the coherence patterns for events 9 and 12 are very similar, despite the obvious differences in Figure 8 between how the amplitudes vary with time for these two events. In the first time window for both events there is good coherence for frequencies up to 2 Hz, but for the remainder of the wave trains there is significant coherence only for frequencies below 1 Hz. A feature of the first part of the wave trains is that the angles of incidence at the base of the basin will have been close to vertical. Consequently, the effective wavelengths of the waves projected onto the Earth’s surface will have been significantly longer than the actual wavelengths, but by uncertain amounts, and, as a result, it is necessary to use the later parts of the wavetrains, after the basic S-wave arrivals, to estimate the number of wavelengths along the Earth’s surface over which the incident wavefields are coherent. Taking the average separation of the firm sites to be 400-450 m, the S wave velocity of 850 m/s in the basement, and the upper frequency for coherence at the basin to obtain the exact incident wavefield everywhere beneath the basin. As it has turned out, for none of the observed events did the incident wavefield even remotely resemble this ideal case, and, given the uniform incoherence of all the observed incident fields, in hindsight it appears that there were unlikely ever to have been ideally-coherent weak-motion events at Alfredton. Thus, the experiment was unsuccessful in providing data to test the 3-dimensional modelling definitively. Even so, the modelling has been successful in reproducing many of the essential-statistical features of the observations, as we will illustrate now.

Our Riccati equation approach works by simultaneously calculating the frequency-domain responses of the basin for a fundamental set of incident waves. Any incident wavefield can be constructed by summing multiples of the fundamental incident fields, and the corresponding response of the basin is obtained by summing the responses for the fundamental incident fields in exactly the same way. The fundamental incident wavefields we have used in modelling the response of Alfredton basin to incident S waves are plane waves with both SH and SV polarizations. Figures 11 and 12 show the amplitude of the surface ground response for a suite of incident SH waves propagating from the east with frequencies 1 Hz, 3 Hz, 5 Hz and 7 Hz. The responses for SV waves coming from the same direction are similar in character except that the east-west (Ux) component of the response is dominant, whereas for the SH waves the north-south (Uy) component is the largest.

Features to be noted include the following. In these and the following three figures of the same type, all three components of motion are shown, including the vertical component (Uz) as well as the horizontal components (Ux and Uy). For the

**FIGURE 9.** Coherence versus frequency, for 6-seconds-long segments of the wavefields at the firm sites, for events 9 and 12. The first segment starts 1 second before the arrival of the S waves.

The spectral ratios in Figure 10 for the north-south line of sites A01, A02, A03 and A04, are the final observational data we will present here. It can be seen that the spectral ratios have similar characteristics to the peak ground displacements in Figure 7; that is, the spectral ratios for the east-west component for the three nearby events 8, 9 and 15 are similar to the spectral ratios for both the east-west and north-south components for the more distant events, whereas the spectral ratios for the north-south component for the nearby events are generally larger. As well, it can be seen that the spectral ratios for the vertical component for three of the four sites shown are comparable in size to the ratios for the other components. For the two horizontal components the largest values of spectral ratio, of between 2 and 6 for the four, representative sites, are for frequencies in the range 3-4 Hz.

### 3-DIMENSIONAL MODELLING RESULTS

The 3-dimensional seismic wave modelling was performed using the Riccati equation approach developed originally by Haines (1996) for perfectly-elastic waves and extended to general linear physical systems by Haines and de Hoop (1996). The corresponding computer codes for 2-dimensional weak-motion problems have been verified by comparing results for simple cases of 2-dimensional basins with solutions obtained using other techniques (Benites and Haines, 1991). This has not been possible for 3-dimensional problems, since no comparable computer codes exist for detailed analysis of the linear responses of small-scale basins. Instead, one of the aims of the experiment at Alfredton was to test the 3-dimensional modelling by finding examples among the observations for which the incident wavefields had simple, well-defined forms, that could be precisely interpolated between the firm sites surrounding the basin to obtain the exact incident wavefield everywhere beneath the basin. As it has turned out, for none of the observed events
examples in Figures 11 and 12, though the incident SH waves involve only motion in the northsouth direction, the other two components are activated in the resulting 3-dimensional response of the basin. In every case the amplitudes have been normalised by taking the amplitude of the incident wave to be 1.0. Consequently, to convert these into values that can be compared with the observations, they have to be divided by 2.0, since this is what the amplitude of the motion at the Earth's surface would have been for the SH case in the absence of the basin and surface topography. As a guide to how large the amplitudes are in these figures, the scale on the left-hand side of each plot has been set to the maximum amplitude plus 7.0, with the exception that a fixed value of 9.0 is used for maximum amplitudes below 2.0. For example, the largest amplitude of the north-south component (Uy) for the 3 Hz case in Figure 11 is approximately 12.0, which plus 7.0 is equal to 19.0 on the scale. After dividing by 2.0 to compare it with the observations, it becomes approximately 6.0. This is very similar to the largest values of the observed spectral ratios at 3 Hz in Figure 10.

Overall, the amplitudes of the north-south component in Figures 11 and 12 are comparable to the spectral ratios in Figure 10, and especially to those for the nearby events. At 1 Hz the presence of the basin has only a small influence on the amplitudes at the surface in Figure 11 which are close to 2.0 everywhere for the north-south component. This coincides with the observed spectral ratios all being close to 1.0 at 1 Hz in Figure 10. At 3 Hz there is a broad, continuous area where there is amplification, with the largest amplitudes being in the centre of the basin where sites A01 and A02 in Figure 10 are located (Figure 2). The largest amplitudes are slightly less at 5 Hz and 7 Hz in Figure 12, and the areas where amplification occurs are less continuous.

The remaining three figures of this type are all for wavefields at 3 Hz. Figure 13 shows the amplitudes of the responses for the SH waves incident from the south and the north the same angle of incidence as for the 3 Hz SH waves incident from the east in Figure 11. The major component of motion in these two
FIGURE 11. Amplitude distributions at the ground surface of the east-west (Ux), north-south (Uy) and vertical (Uz) components of
the fundamental wavefields responding to incident Love waves of 1 Hz (top) and plane incident SH waves of 3 Hz (bottom) with an
incident angle of 25°. Both incident wavefields propagate from the east, and the amplitudes, here and in Figures 12 to 15, are
normalised with respect to the amplitudes of the incident waves, which for SH waves in the idealisation of a flat uniform halfspace,
are half the size of the total wavefields at the surface. The wavenumbers (kx, ky) in the heading for each set of plots give how many
cycles of the incident waves there are across the base of the area where the wavefield has been modelled. Negative and positive values
of kx indicate propagation from the east and west respectively, while negative values of ky indicate propagation from the south and
positive values indicate propagation from the north.
FIGURE 12. Amplitude distributions at the ground surface of the fundamental wavefields responding to plane incident SH waves of 5 Hz (top) and 7 Hz (bottom) propagating from the east with incident angles of 15° and 10° respectively.
FIGURE 13. Amplitude distributions at the ground surface of the fundamental wavefields responding to plane incident SH waves of 3 Hz propagating from the south (top) and north (bottom) with the same incident angle of 25°.
cases is in the east-west direction. Otherwise, the patterns are broadly similar to that in Figure 11, but at any given point the amplitude of the major component has different values in all three cases. A feature to note in all these examples is that the amplitudes are low at the southern end of the basin because the basin has been artificially terminated there, as is shown in Figure 5. In Figures 11 and 13 the angle of incidence for the 3 Hz waves is 25° to the vertical. The incident waves for the cases in Figure 14 are horizontally propagating from the same geographical directions as in Figure 13. At most points the amplitudes in Figure 14 are much smaller than for the cases in Figure 11 and 13.

In computing the response inside the basin, the wavefield has been expressed as the superposition of depth-dependent contributions from a preset number of horizontal wavenumbers. For the results presented thus far there were 69 horizontal wavenumbers. The number of horizontal wavenumbers determines how long the computations take. As well, there is spatial smoothing inherent in using expansions in terms of wavenumbers. 69 was chosen as the minimum number that would give an adequate representation of the variability of the wavefield between different sites. With this number of horizontal wavenumbers, the wavefields and the geometrical and physical parameters of the layers in the basin are averaged horizontally within each layer over an area approximately of radius 70 m about each point – in the calculations using the Riccati equation approach, there is no smoothing in the vertical direction. This horizontal resolution is sufficient to give separate results for each recording site, except where the recording sites were only 50 m apart, and is comparable to the spacing in Figure 3 at which probing was performed to determine the properties of the basin. Figure 15 shows the amplitudes of the surface ground response for the same 3 Hz incident SH wavefield as in Figure 11, except that 109 (top) and 145 (bottom) horizontal wavenumbers have been used. In these cases the approximate radii for the horizontal averaging are 60 m and 50 m respectively, and the computations took roughly 4 and 9 times as long. The patterns in Figure 15 are very similar to that in Figure 11, though the extra smoothing in Figure 11 is clearly evident. The time-domain wavefields described below were computed using the set of 69 horizontal wavenumbers, for the obvious reason of computation time, given the huge number of calculations involved.

To provide modelling results that can be compared directly with, for example, the peak motion statistics in Figure 7 for the observations, suites of synthetic “earthquakes”, for which ground motions were computed in the basin and at the surrounding firm sites, were generated in the following manner. First, solutions were obtained for the wavefields in and around the basin generated by the 69 different SH waves and 69 different SV waves in the fundamental set of incident wavefields for each of 120 uniformly spaced frequencies up to 12 Hz (giving 10 seconds of synthetic record for the comparisons) – the number of incident wavefields in each fundamental set, which includes P waves as well, matches the number of independent 3-dimensional responses calculated for the basin, which is determined by the number of horizontal wavenumbers used in computing the wavefields. In other words, 16,560 independent, 3-dimensional, single-frequency, 3-component wavefield solutions, like the 8 in Figures 11 to 14, were computed for incident S waves.

Next, to mimic the incident waves for an actual earthquake, randomly chosen multiplying factors, or weights, were selected for the incident wavefields in the fundamental set for each frequency. The weights were chosen so that the composite incident wavefield obtained by summing all the contributions would have similar spatial coherency properties (Figure 9), angles of incidence and time histories, to a given set of observed ground motions at the firm reference sites, whose time envelopes and frequency spectra were used as templates in generating the random numbers used as the weights. Then, the responses for all the fundamental incident wavefields were summed, multiplied by these weights, to give the total time-domain response for the synthetic earthquake. This was repeated several times, keeping the same observed earthquakes as models, with the result that different synthetic events with similar time histories were generated with randomly different combinations of SH and SV waves and combinations of azimuths of arrival. For any given synthetic event the displacement paths of the ground motions for the incident waves were random functions of time, just like the observed displacement paths in Figure 8.

Figure 16 summarizes the normalised east-west and north-south peak ground displacements at individual sites for two sets of synthetic events, in the same way that Figure 7 summarizes the observations. The normalisation for the synthetic results was with respect to the RMS of the peak ground displacements at the four basement sites in Figure 16 closest to the reference sites for the observations A00, A05, A11 and A12. One set of synthetic events all have time envelopes similar to that in Figure 8 for the largest nearby event 9, with the frequency contents also being much the same as for event 9. The other set are similar to event 12, which is also shown in Figure 8 and had the largest ground motions of the more distant events. For the synthetic “nearby” events with short durations of the incident S waves, the normalised peak horizontal displacements in the basin are generally larger than for the synthetic “more distant” events with longer durations, reflecting the fact that for short-duration events there is less opportunity (i.e. a smaller window in time and space) for the incoherent wave packets inside the basin to overlap and destructively interfere. The patterns and values in Figure 16 are clearly similar to those in Figure 7, except that in Figure 7 the amplitudes for the nearby and more distant earthquakes are noticeably different only for the north-south component. As already remarked, this might have been a chance occurrence, with all the nearby events being almost identical events that happened to give larger amplitudes in the basin in the north-south direction than in the east-west direction.

The individual synthetic “nearby” events, on the other hand, have quite different incident wavefields, effectively covering a wide range of possible source mechanisms, with the result that on average the amplitudes are similar in the north-south and east-west directions.

**CONCLUSIONS**

The 3-dimensional modelling confirms that in the Alfredton basin the peak ground motions are more amplified for events with short duration incident wavefields than for longer duration events. This result appears to be an interesting curiosity, rather than having a direct bearing on seismic hazard, since 3.7 was the magnitude of the largest nearby earthquake for which the ground motion was of short duration. For large nearby events the duration of the ground motion is likely to be much longer, and, consequently, the amplification of the peak ground motions may be more like that observed and modelled for more distant events. This is not, however, a firmly based conclusion, as it is uncertain whether for earthquakes large enough to be relevant in assessing seismic hazard, the incident wavefields will be as
FIGURE 14. Amplitude distributions at the ground surface of the fundamental wavefields responding to incident Love waves of 3 Hz propagating from the south (top) and north (bottom).
FIGURE 15. Amplitude distributions at the ground surface of the fundamental wavefields responding to the same plane incident SH waves of 3 Hz as in Figure 11 (bottom), where the solution in the basin has been obtained with 69 horizontal wavenumbers. Here 109 (top) and 145 (bottom) horizontal wavenumbers have been used instead.
spatially incoherent as those for the weak-motion events we have observed and modelled. In other words, we have no data on what the coherency properties of the incident wavefields will be like at Alfredton for a large earthquake. On the top of this is the question of nonlinear behaviour of the sediments in the basin in earthquakes that are large enough. Another area of uncertainty is whether the amplifications for shallow earthquakes which excite significant surface waves, will be larger than from the lower crustal and deeper subduction-zone events that were observed. In this case, we can infer from the modelling that larger amplifications from shallow earthquakes are unlikely at Alfredton, as is illustrated by the low amplifications for surface waves in Figure 14.

Both the observations and the modelling show that the amplifications of the peak motions and the Fourier spectra of the motions in the basin at Alfredton are smaller than in situations where there are large abrupt changes in shear-wave impedance to trap seismic energy in the low-velocity soft-soil layers. In particular, because of the lack of any such boundary to trap the energy, the duration of the ground motion is not noticeably longer in the basin than at the surrounding firm sites. The lesson from this is that the presence of an appreciable thickness of soft soil is not necessarily indicative of a major seismic hazard comparable to that in Mexico City, unless there are large contrasts in shear-wave impedance at the boundaries between soil layers or between the soft soil and the underlying basement rock.

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This paper has been written because of the need to publicise the modelling results, which would have been meaningless without presenting sufficient of the observational data to place the modelling into context. John Haines is especially grateful to Jiashun for allowing him to do so at the same time as Jiashun is completing his thesis. The data that are presented here have been selected to emphasise the incoherent nature of the incident wavefields and to give an indication of the character of the observed amplifications, and the interpretations are focussed on explaining why the modelling took the form that it did. The presentation by Jiashun in his thesis is a complete analysis, which he will be presenting in subsequent papers.

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